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GF Steel-Tile An Economical System of Floor Construction



The General Fireproofing Co Youngstown, Ohio



GF STEEL-TILE

AN ECONOMICAL SYSTEM OF FLOOR CONSTRUCTION

A Statement of the Many Accepted Advantages of "T" Beam Floor Construction, with Particular Reference to the Use of Steel-Tile-a Collection of Tables for Designing and Building Steel-Tile Floors-a Complete Specification for the Work from Start to Finish



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Steel-Tile Floors

An Economical System of Floor Construction

F STEEL-TILE are in reality no more nor less than permanent steel forms for concrete floors daried by the steel forms for concrete floors daried by the steel floors daried by the stee steel forms for concrete floors designed on the T beam system -small beams or joists closely spaced, with a thin connecting slab of concrete.

This type of construction is recognized as particularly good on all long span work in such structures as schools, apartments, hotels, office buildings, lofts, warehouses and stores. Steel-Tile floors can be constructed at a substantial saving in cost and with absolute safety pre-determined. It does not matter whether the frame is of steel or of concrete-Steel-Tile are used to equal advantage in both types.

This method gives an exceptionally light floor and because the Steel-Tile, which are actually forms, carry all the concrete except the narrow joist, they save a great part of the usual cost of form work and centering.



Terra-Cotta tile and concrete floors, while built upon the same principle, that is, the T beam principle, are not to be compared with Steel-Tile floor construction. In the matter of dead load alone, the necessary Steel-Tile for a job will weigh only about 10% of the required Terra-Cotta tile, and the finished floor, of course, will show a proportionate saving with Steel-Tile. Add to that the reduced freight charges, lower handling expense on the job, and saving on the supporting members of the building, and the fact that Steel-Tile joists are spaced at least 24" on center as compared with 16" for Terra-Cotta tile, GF Steel-Tile floors are decidedly superior.

The flat slab reinforced concrete floor is also being displaced by Steel-Tile construction in many types of buildings. The chief saving here is in weight, for Steel-Tile forms eliminate a great part of the dead concrete with which such floors are burdened. The deep narrow joist in Steel-Tile floors have practically no useless material or dead load in them.



It is scarcely necessary to mention the superiority of Steel-Tile over non-fireproof construction. Fireproofness, with its resulting lower insurance rates and the total absence of up-keep expense, are enough to prove the case, even without the greater value of safety to life and property.

Ceilings, when applied beneath Steel-Tile floors, are uniformly smooth and never show up streaky as is often the case when other forms of tile are used.

The peculiar economies of Steel-Tile floors mentioned above and following in greater detail, have been combined with complete specifications and tables in order to give Architects, Contractors and Engineers a comprehensive knowledge of this most economical floor construction system, as briefly as possible.

GF Steel-Tile Floors are Light in Weight

Consider the fact that GF Steel-Tile occupy from 45% to 60% of the cubical contents of a floor. Where formerly this space was filled with lazy concrete or heavy tile, Steel-Tile transforms it into dead air space weighing nothing. Yet this is done without sacrificing anything in the strength of the floor.

Such a large reduction in actual weight permits lighter girders, lighter walls, in fact, lighter construction all the way through to the very footings

of the structure.

They Are Safe

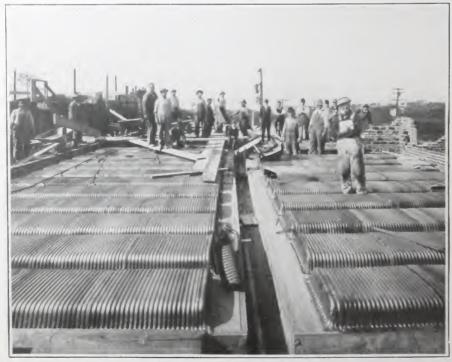
Less weight does not in the least imply a sacrifice in safety. On the contrary every pound of concrete in Steel-Tile floors is of use and so built and graded from thin slabs to deep reinforced joists that its whole strength can be exerted in sustaining any live loads placed upon the floor.

From accurate tables on pages 12 to 20 inclusive, the necessary measurements are given for Steel-Tile floors which must support given loads

over any of the spans most commonly encountered.

They Are Economical

Note again the proportion of Steel-Tile floor contents that is simply air space—45% to 60%. Here is a clear saving in material that means a



McSorley Duplex Apts., Pittsburgh, Pa.
Architects—Perry & Thomas, Chicago, Iil.

J. McSorley, Pittsburgh, Pa., Owner and Builder

greatly reduced cost at the very outset. And that is not the only saving. Solid steel forms which lap tightly at both ends and are joined by a simple but effective centering on the sides, prevent any leakage and waste of concrete. The same forms made, as they are, from sheet steel cold pressed to shape, are transported cheaply and without danger of breaking. The laying of Steel-Tile is a simple matter quickly accomplished.

On pages 7 to 11 the form work or centering for Steel-Tile floors is shown. Notice that it is a simple skeleton frame with runners along under the line of the joists, with the intervening space left entirely open. Such form work requires much less lumber than the solid type used for floors of the solid concrete slab or tile block design, and less time is required to erect and tear them down.

They Are Quickly Laid

GF Steel-Tile can be laid more rapidly than any other form of fireproof floor construction. With the centering up, the Steel-Tile and reinforcing bars for the joists are easily set in place. Sometimes spacers are used, but generally it is only necessary to nail the Steel-Tile lightly to the centering before pouring the concrete. End-Tile are set at the end of each row clos-



ing up the form and leaving the proper amount of space for concrete to take up any shearing tendency near the beam or girders. With two standard lengths of Steel-Tile—30" and 35"—little time is lost in match-

ing at the ends of odd length spans.

GF Steel-Tile are furnished immediately from stock in 6", 8", 10" and 12" heights and in 30" and 35" lengths. End-Tile to match are also carried for prompt shipment. These exclusive features of Steel-Tile floor construction and the more general advantages as outlined, warrant the consideration of Steel-Tile for almost every building operation. Interesting cost data and valuable advice will be furnished by The General Fireproofing Company if you will give them the necessary figures for your work.

Ceiling Construction With GF Steel-Tile

Flat ceilings and smooth surfaces are obtained with Steel-Tile floor construction.

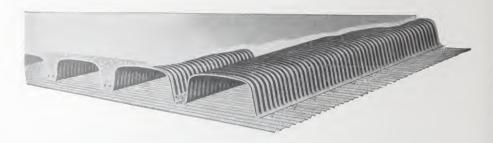
Beams and girders are made of the same depth as the narrow joists and Herringbone Rigid Metal Lath is run continuously over the entire ceiling. Or, when walls occur, a smooth, clean angle or cove is easily constructed.

Two methods are in common use for ceiling construction.

The more practical method for all purposes is to finish the Steel-Tile floor before erecting any of the ceiling lath. Tie wires are run through the holes in the Steel-Tile and extended below the finished joist.

³/₄" GF Cold Rolled Channels are securely wired to the joists and the Herringbone erected in the usual manner beneath the channels.

When such a job is finished there is no possibility of streaking, for the ceiling plaster does not come in contact with the deep floor joists which dry out more slowly than the thinner ceiling and floor slabs. The surface is perfectly smooth and should a suspended ceiling be required, it can be easily put up by extending steel suspension rods down the required distance and erecting the channels and Herringbone as before. See page 10.



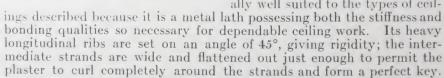
The second and simpler method is to place the Herringbone on top of the centering and lay the Steel-Tile directly on it. After the centering is removed the ceiling is plaster finished.

This method results in a very economical ceiling which can be depended upon to remain solidly in place and give good service. There is a direct

saving of the cost of the channels plus a greatly reduced cost for erecting.

The type of Herringbone recommended for ceiling work, where lath is fastened directly to the underside of joists formed by Steel-Tile, is AAA 24 gauge, which will easily span 20 inches. In channel-funed ceilings, AAA 27 gauge Herringbone should be used, channels to be placed on 16-inch centers.

AAA Herringbone is exceptionally well suited to the types of ceil-



Specify "Herringbone" Rigid Metal Lath for use with your Steel-Tile floors. The resulting job, top, bottom and all the way through, will be a source of complete satisfaction. Other uses for Herringbone are described at length in a new book which is furnished on request.



"AAA" Herringbone

General Specifications for Reinforced Concrete

Additional Copies Furnished on Request

The floors and roofs shall consist of the GF Steel-Tile system of reinforced concrete construction, as shown on the accompanying plans, and all materials and workmanship shall be in strict accordance with these plans and specifications.

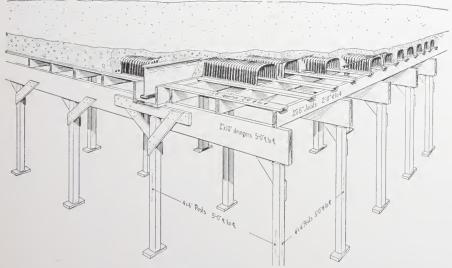
The Contractor shall, at all times during the progress of the work, provide a competent foreman who is thoroughly experienced in reinforced concrete construction, whose duty it will be to see that these plans and specifications are carried out. The Architect reserves the right, at any time, to discharge any incompetent or careless employee and such employee shall not be reinstated upon the work without special permission in writing from the Architect. The Architect, or his authorized representative, shall at all times have complete access to the work and the Contractor shall place at his disposal every facility for the inspection of material and workmanship.

Materials and Workmanship

The object of these specifications is to provide a first class structure and all work shall be done in a thorough and businesslike manner. All materials shall be in strict accordance with these specifications and any materials rejected by the Architect must be immediately removed from the vicinity of the work.

Cement

The cement used in this work shall be Portland Cement and must conform to the standard specifications of the American Society for Testing Materials. All cement shall be tested as directed by the Architect before being brought to the vicinity of the work, and the Contractor must provide ample time for performing these tests so that no delay in the work will be occasioned.



GF Steel-Tile Floor Construction, showing Typical Form Work

Sand

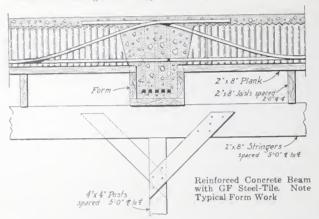
The sand used for the concrete shall be sharp graded bank or lake sand screened to pass through a 1/4" screen and proportionately graded from fine to coarse, with the coarse grain predominating. All sand shall be free from loam, vegetable or other injurious matter and it shall contain not more than 4% of clay.

Stone and Gravel

The stone shall be clean crushed stone reasonably free from crusher dust, and free from loam, vegetable or other injurious matter.

The gravel shall be washed clean.

Both stone and gravel shall pass through a 1" screen and be retained on a 1/4" screen.



Steel-Tile

The Steel-Tile shall be GF Steel-Tile as manufactured by The General Fireproofing Company of Youngstown, Ohio. The Tile shall be of the sizes indicated on the drawings and must be used strictly in accordance with these specifications. The Steel-Tile shall be accurately spaced to secure the joist area called for and must be

tacked securely to the centering with light nails to prevent movement while the concrete is being poured. The ceiling hangers must be placed and properly adjusted according to detail before the concrete work is commenced.

Reinforcing Steel

The reinforcing steel shall comply with the standard specifications of the American Steel Manufacturers Association. All reinforcing members shall be accurately located in the forms and secured firmly against displacement. They shall have a protection of concrete or cement mortar not less than 2" thick for hooped or plain reinforced columns and $1\frac{1}{2}$ " thick on the bottom and sides of girders and beams, $\frac{5}{8}$ " on the bottom of floor slabs, and $1\frac{1}{2}$ " on the bottom of Steel-Tile joists.

Proportion and Placing of Concrete

All concrete shall be mixed in proportion of 1 cu. ft. of cement, 2 cu. ft. of sand and 4 cu. ft. of stone. One barrel of cement shall be considered as 3.8 cu. ft. by volume. Before pouring concrete, each piece of the steel reinforcement must be thoroughly fastened in its proper place and must be held there until the pouring is completed. Concrete shall be mixed by an approved batch mixer, and must be conveyed to place in such manner that no separation of the ingredients occurs. Concrete shall be deposited before the initial set takes place and the work shall be so laid out that partially set concrete will not be disturbed by trucking or wheeling over it.

When concreting is once started, it shall be carried on as a continuous operation until the pouring of the section or panel is completed. If the concreting should be stopped, care must

be taken to stop the work at such a point that joints formed when the work is resumed will not weaken the members structurally.

All columns are to be filled at least three hours ahead of the floor construction to allow the concrete in the column to properly set up. The filling of the column must be in one continuous operation to the level of the bottom of the girder or beam supported by it.

In pouring columns the concrete is to be kept well stirred or puddled with a long pole or rod to prevent voids and honey-combing; filling the columns completely and puddling afterwards will not be allowed.

All beams shall be poured so as to be monolithic with the adjacent slab, that is, poured continuously from the bottom of the beam to the top of the slab. When fresh concrete joins concrete that is set or partially set, the exposed surface of the old concrete shall be thoroughly cleaned and be given a grout coating of neat cement before any concrete is poured.

Concrete laid during hot weather shall be thoroughly wet with clean water and be continually moistened during the first seven days after placing.

Concrete work shall not be permitted when the temperature is 32° Fahrenheit or less unless sufficient precaution is taken to prevent the concrete from freezing after having been put in place. No frozen materials shall be used.

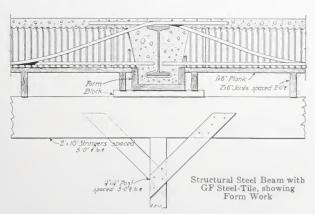
Extreme care must be taken in the removal of forms under concrete that has been frozen, and forms shall not be removed until it is assured that the moisture has left the concrete and it has obtained its permanent set.

Forms

All forms shall be strong and rigid and sufficiently watertight to prevent leakage of mortar. Care should be taken to insure that all debris is removed from forms and that they are thoroughly wetted before concrete is deposited in them. Column forms shall be so designed that they may be removed without disturbing the beam and slab forms, and cleanout holes shall be provided in the bottom when necessary to insure the removal of wood chips or other debris. Beam forms shall be so designed that the sides may be removed without disturbing

the bottom, and on long spans this shall be given a slight camber to take care of unavoidable settlement when pouring the concrete.

The time for the removal of forms shall vary with the design and with the temperature. Twenty-one days of good drying weather with a temperature above 60° shall be taken as the standard for the removal of forms carrying dead load, and three days of good drying weather with a temperature above



60° shall be taken as the standard for the removal of vertical forms carrying no dead weight. Beams and girders of 25-foot span or over shall be considered as special cases and shall be subject to the inspection of the Superintendent before removal of the support.

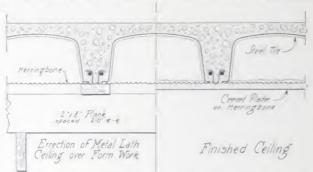
All reinforced concrete shall be carefully inspected to insure its soundness and reliability before main supports are removed.

Special care shall be taken on the removal of forms under concrete that has set and cured during freezing weather. Concrete which has been accidentally frozen during the process

of setting shall be thawed out and kept heated until it is assured that the concrete has thoroughly set. Sufficient water shall be added to the concrete during the process of thawing and setting to insure the hydration of the cement.

Loading Tests

The Contractor shall at his own expense provide sufficient material and labor to make not more than two loading tests to such portions of the building as the Architect may select. Said tests must be made within a reasonable time after the forms are removed, and must show that the floors are capable of sustaining twice the figured live load without cracking or undue



Herringbone Ceiling Erected Directly on Form Work (See also page 6)

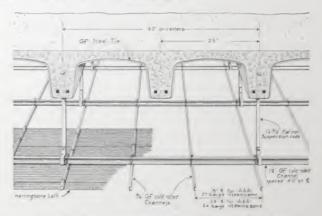
deflection. One month of good drying weather after removal of the forms will be taken as the proper time for the making of such tests.

Metal Lath Ceiling

Before the concrete is poured, No. 14 Gauge tie wires shall be placed

through the joists in pairs at approximately 15%4" on centers and of sufficient length to take the supporting channels for the ceiling. After the reinforced concrete is thoroughly set, the centering shall be removed and the metal lath ceiling erected.

3/4" GF Channel Furring shall be placed approximately 153/4" on centers, shall be securely



Method of Erecting a Suspended Ceiling Under GF Steel-Tile

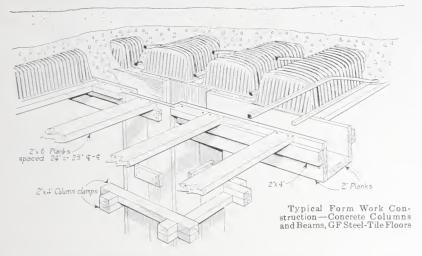
fastened by means of the 14 Gauge wires and leveled up to the proper elevation. To this shall be wired AAA 27 Gauge Herringbone Lath, as manufactured by The General Fire-proofing Company, and this Lath shall be fastened in a thorough workmanlike manner before plastering is begun.

Alternate Metal Lath Ceiling Construction

When the forms are completed and before the placing of the Steel-Tile, AAA 24 Gauge Herringbone Lath shall be placed over the forms with the ribs running at right angles to the line of the joist. The Steel-Tile and reinforcing steel shall then be placed and the Herringbone Lath wired to the reinforcing steel with 14 Gauge wire at intervals of 9". The concrete shall then be poured.

Suspended Ceiling

Where a suspended ceiling is required, $1'' \times \frac{3}{16}''$ flat hangers shall be suspended from the concrete joists spaced about 4 feet on centers, as shown on the accompanying detail. After the concrete floor is completed 1'' or $1\frac{1}{2}''$ GF Cold Rolled Steel Channels shall be fastened to the bottom of the hangers and properly leveled. When this is done, wire $\frac{3}{4}''$ GF Cold Rolled Steel Channels to the underside of the $\frac{1}{2}''$ channels, spacing them $\frac{2}{4}''$ on centers and under these wire securely AAA $\frac{2}{4}$ Gauge Herringbone Lath with the ribs running at right angles to the line of the $\frac{3}{4}''$ channels.





State Armory, Akron, Ohio. Karl I. Best, state architect Contractors, Clemmer & Johnson, Hicksville, Ohio

Explanations of Steel-Tile Tables

The accompanying tables are tables for safe live loads in pounds per square foot for the GT Steel-Tile Floor System. The weight of the floor slab has been deducted from the loads as given. Any additional dead loads, such as the weight of the ceiling construction or the finalized floors, should also be deducted before the safe live load is obtained

The stresses is the steel and concrete are limited to 16,000 lbs, per square inch, and 650 lbs, per square inch respectively, and the fireproofing of the reinforcement has been fixed at 1 ½" from the bottom of the joist, and 1" from the side of the joist. The distance center to center of the steel bars in each joist should never be less than two and one-half times the manifold diameter or side of har. This spacing allows the concrete to flow freely around the reinforcing and also matres sufficient concrete to transmit the stresses from the steel to the T section of the beans.

The mass of the bors at shown in this table are all for square bars, but round bars can very readily be substituted by reference to the tables of areas on page 30 of this Handbook.

The lowery lines shown in these tables are drawn with a vertical shearing force producing an average shearing atress of 60 lbs, per square inch on the concrete. In all cases, above and to the right of this line, the additional web shearing stresses should be cared for by using stresses, or other sailable reinforcement.

A small amount of additional steel should always be placed in the slab at right angles to the line of the post to present crocks in the concrete, due to contraction during process of setting, and to low temperatures. Agree of the floor surfaces, are commonly used for this purpose.

EXAMPLE

Lat it be required to design a Steel-Tile floor with a span of 20 ft, to support a net live load of 80 lbs, per sq. It. If the weight of the colling be assumed at 10 lbs, per square foot, and the weight of the floor family at 10 lbs, per sq. It. According to the tables, it is found that the following designs would be suitable:

Star	Bara	Strength	Dead Weight
8" Steel-Tile plus 2" enneset	a 75" an plus 1," sq.	107 lbs. per sq ft.	55 lbs. per sq.ft.
10" Steel Tile pine 2" concret			52 lbs per sq ft
12" Sheel-Tile plus 2" comeret	e da" sq plus da" sq.	106 lbs. per sq.ft.	70 lbs. per sq.ft.

By abserting the shear line of the table, it is seen that if the 8 plus 2 or the 10 plus 2 slab is chosen, the shearing streams in the web will be cared for by the use of stirrups, but if the 12 plus 2 slab is chosen, no stirrups will be needed. If stirrups are used, they can be calculated by reference to formula 1 on page 24. For economical design, the designer will compute the control of the Stool-Tile, concrete and reinforcing steel in place for each thickness of slab and select the one whose sum is the least. The form work need not be taken into consideration.

NOTE: ON PAGE 27 A TYPICAL STEEL-TILE FLOOR COMPUTATION IS GIVEN TO ILLUSTRATE THE USE OF THE COMMON FORMULAS FOR REINFORCED CONCRETE DESIGN

GF Steel-Tile Table of Safe Live Loads in Pounds per Sq. Ft.

	Depth			6" 1	TLE+	"CONC	RETE		
		W	eight of S	ists 21' Slab and per Sq. F	bs.	5" Joists 25" C-C Weight of Slab and Joist 48 lbs, per Sq. Ft.			
	Area of Steel	Sq. 1n.	.39 Sq. 1n.		Sq. In.	78 Sq. In.	.95 Sq. In.	1 12 Sq. In.	1,34 Sq. In
Sq	Size of quare Bars	-12"	28/2	12" + 12"	+ 13	100	34" + 56"	34"+34"	1 8 + 11-2
R. M.	(In Ft. Lbs.)	1030	1510	5000	2520	3040	3500	¥100	1110
	10 11 12	57 39	111 84 63	15 k 119 93	206 162 129	258 206 166	191 515 305	362 201 236	396 318 260
	13 14		47 34	72 56	103 83	134	159 130	194	178
æ t	15 16			13	66 52	89 73	107 89	184	149 125
in F	17 18				14	59 48	78 60	93 78	105 89
рап	19			1		38	4.9	66	75
Length of Span in Feet	20 20						39	55 45	68 58
Lengt	51 53 55							37	43 35
	25 26 27 28								

	Depth			8"	TLE +	2" CONC	RETE	
		5" Jot	STS 25"	C-C	Weight	of Slab at	id Joist 5	5 lbs. per Sq. F
	Area of Steel		.64 Sq. In.	-78 Sq. In.	Sq. In.	1 18 Sq. In.	1,34 Sq. In.	1.53 Sq. In.
Size of Square Bars		12"+12"	55"+12"	10 10 10 10 10 10 10 10 10 10 10 10 10 1	1,4 1,4	34"+34"	+ 25	1.57 + 7.57
R. M.	(In Ft. Lbs.)	2540	3230	3900	4700	5510	6500	6900
	10	199	268	335	415	496	595	635
	11 12 13	155 121 95	212 169 135	267 216 176	333 271 223	400 328 271	481 395 329	515 425 353
	14	75	110	143	185	558	276	297
-	15	58	88	118	154	190	233	198
in Feet	16 17 18	44 33	71 56 45	97 80 65	199 107 90	160 135 115	199 169 145	215 184 158
Span in	19		34	53	75	97	125	136
Length of Sp	55 51 50			48	68 88 13	83 70 59	107 92 79	117 101 87
	23 24				84	4.9 4.1	67 57	75 65
	25 26				1	33	49	55 47
	27						31	39
	28				1			33

	GF	Steel	l-Tile			
Table of Safe	Live	Loads i	in Pounds	per	Sq.	Ft.

D	epth			10"	TILE +	s" CONC	RETE		
		5" Jo	I-T- 25"	C-C V		Slab and	Joist 62	lbs. per S	q. Ft.
	rea of Steel	.50 Sq. In.	64 Sq. In.	-78 Sq. In.	.95 Sq. In.	1.12 Sq. In.	1_34 Sq. In.	1 53 Sq. In.	1.76 Sq. In.
Size of Square Bars		12"+19"	6 8 " + 19"	ь к · + ь к · ·	8 4" + 5 K"	n4 " + h4 "	78"+8,"	" " + " " x L	"x2 + "1
R. M. (In Ft. Lbs.		3150	4000	4850	5870	6910	8190	9320	9450
	10	256	338	423	525				
	11 12 13	200 158 126	268 216 175	338 274 225	423 345 286	510 418 348	506 423	490	498
	14	100	142	186	237	291	356	414	151
-	15	79	116	153	199	245	301	353	348
Length of Span in Beet	16 17 18 19	62 48 37	94 76 61 49	127 106 88 72	168 141 119 101	208 178 152 130	258 221 191 165	302 260 226 196	308 266 230 200
of Si	20 21 22		38	59 45 35	\$5 71 59	111 95 81	143 124 108	171 1.50 130	174 152 133
Len	23 24				4.9	69 55	93 80	114 100	117 102
	25					49 40	69 59	87 76	89 78
	27 28 20						50 42	66 57 49	68 59 50
	30.							45	43

Hepth				12" TILE	E+5, C	ONCRET	T.F		
	5	"Јонть	25" C-C	Weig	bt of Sla	b and Joi	st 70 lbs	per sq. I	Ft.
Area of Stood	30 Sq. In.	Sq. In.	78 Sq. In.	95 Sq. In.	1 12 Sq. In.	1_34 5q. In.	1.55 Sq. In.	1 76 50. In	5 00
Some of Square Bars	5"+ 5"	0 N V + 12 V	1 N N N N N N N N N N N N N N N N N N N	3 + 1 B 1/2	" + " + " t	Cq. ' + 8, ''	Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z		1+1
R. M. (In Fi. Line)	3840	4830	5850	7020	5350	9570	11240	11700	12900
Tength of Spina in Feet 115 114 115 115 115 115 115 115 115 115	514 248 196 137 180 100 89 63 45 89	612 288 264 215 170 144 118 97 78 63	515 41.5 13.6 27.6 22.5 19.0 15.8 13.2 14.0 52 75, 62 540	514 420 147 200 243 200 174 145 125 106 90 70 53	510 424 355 100 257 219 187 101 129 119 102 85 75	515 455 970 910 214 201 177 154 134 116 101 86 76 66	505 480 470 320 278 242 211 188 162 142 142 145 165 80	527 450 981 285 284 284 282 100 172 152 130 147 105 90	477 411 55 110 271 285 200 114 162 114 127 112 97

	GF	Steel-7	Γile		
Table of Safe	Live	Loads in	Pounds	per Sq.	Ft.

	Depth	6" TILE + 2½" CONCRETE									
			Weight	of Slab a bs. per S	5" JOISTS 25" C-C Weight of Slab and Joist 54 lbs. per Sq. Ft.						
	Area of Steel	.25 Sq. In.	.39 Sq. In.	.50 Sq. In.	.64 Sq. In.	.78 Sq. In.	.95 Sq. In.	1.12 Sq. In.	1.34 Sq. In.	1.53 Sq. In	
Se	Size of quare Bars		18/2	1/2" + 1/2"	5/8" + 1/2"	24"+6%"	34"+5%"	34" + 34"	78"+34"	"%"+"%"	
R. M.	(In Ft. Lbs.)	1110	1700	2170	2720	3590	3780	4420	5180	5410	
	10 11 12 13	59 40	118 89 66 49	165 127 98 76	220 173 137 109	277 220 176 143	324 258 208 169	388 311 252 208	464 364 306 252	487 389 321 266	
	14		35	58	87	116	139	172	210	222	
set	15 16			44	69 54	94 77	114 93	142 118	176 148	186 158	
in E	17 18				42	62 50	76 62	99 82	125 106	133 113	
pan	19					39	51	68	89	96	
Length of Span in Feet	20 21					30	40	56 46	75 63	81 69	
Lengt	22 23 24							37	53 44	58 48	
	25 26 27 28 29										
	30										

			Steel-				
Table of	Safe	Live	Loads in	Pounds	per	Sq.	Ft.

	Diepith	10" TILE + 25,0" CONCRUTE								
		3'	Juste	25" C-C	Weig	ht of Sla	b and Joh	of 68 Hu.	per Sq. I	- (
	Area of Sheet	50 Sq. In.	64 Sq. 10,	78 Sq. Iu.	Sq. fr.	1.18 Su. In.	1.54 Sq. In.	1.53 Sq. In.	1 70 Sq. In	8 00 Sq. In
Size of Square Dices		H+ H	W+36	14.4%	355 + 376 376 + 376	100+100	74°+94°	14.44.11	"4 + "4"	1 + 1.1
L M	(In Pt. Lim)	3570	4180	5050	6090	7180	8810	9480	11000	11850
Span to Pred	1.0 1.1 1.9 1.9 1.4 1.6 1.0 1.7 1.7 1.7 1.7	0014 000 100 100 100 100 100 70 04 47 04	950 278 282, 180 240 116 95 71 61 45	101 100 100 101 100 100 100 80 70	488 954 992 242 902 170 138 180 Lon	#44 429 250 598 #50 #18 160 152	5 24 457 907 910 204 287 195	505 4#7 568 #10 #07 #91 #60	494 462 364 515 672 687	510 4 17 176 114 240
Longith of Span	60 61 60 61 61 63 66 67 67 69 79		**	48 - 27 - 27	8.4 7.0 0.7 4.7 9.90	111 94 60 67 57 47 58	1 4.0 T 0.5 T 0.5 T 0.5 0.0 d 0.0 d	174 150 100 112 100 87 73 68 55 47	207 180 160 140 123 108 65 78 65	210 109 100 140 180 144 100 Ne 77 07

	Dayth					0000				
			Lower	35" 414	59 mg	de of Mic	ok hand do	a to the	per riq.	1
	Strat. of Ethni	m_0, L_1	50 Ja	So la	92 94 In	E 12 For Lin	E. Sa No. In	7.89 m ₁ . In.	Sq. fo.	10 to 10
		4	5	2	2	1/2	5	8	1/2	L
	See all		1.0	- 5	4	+		1		1
	coale flare	7.0	7		7.	1	+	2.0	1	1.7
		5	t	+ 19	5	2	8	8	1	1 16
MC	(Le Ft Lim)	1000	8,3000	8070	7,679	8000	10000	rtma	19000	13,600
	10	200	200							
	10	TAX	200	10.00						
	116	399	HOL	342	6500	. AGE				
	13.	3,44	EXE	3000	3,002	471	ERD			
	10	3,8%	1.54	STA	250	1056	850	4334		
	Li.	100	THE	1093	WAN	delice	TTW	4.79	10.54	
	16	74.	1.19	1,000	100	2000	nin.	876	440	460
3	. 11	201	29	1154	15W	101	-107 k	-891	391	411
-	79	- 23	21	XIX	167	1360	630	2004	mer.	- 248
bearth of last or floor	11	7.6	*1	- Sex	1100	130	1009	1044	200	211
2.1	- 0		34	776	THE R	1100	778	70.5	204	254
-	100		377	XX	3195	1.196	138	794	713	1230
2	101			Alt	7%	1.03	3,00	1,825	2.87	HE0
8	23			701	100	9-0	SHE	100	374	706
8	- 0				-87	76	- 65	334	2119	1778
	107				10	48	100	100	2364	3.54
	- 0					39.	-0	107	3300	337
	111					- 63	66	194	110	3399
	-0.					-84	- 68	73	34	7.00
	- 10						6.6	.51	91	.04
							98	38	71	84

	GF Stee	el-Tile		
Table of Safe	Live Loads	in pounds	per !	Sq. Ft.

	Depth					3" CONC			
		We	ight of Sl	24" C- lab and J er Sq. Ft	oist		Weight 60 l	oists 23 of Slab a bs. per S	nd Joist
	Area of Steel	.39 Sq. In.	.50 Sq. In.	.64 Sq. In.	.78 Sq. In.	.95 Sq. In.	1.12 Sq. In.	1.34 Sq. In.	1.53 Sq. In.
	Size of quare Bars	28/2	1/2" + 1/2"	5/8" + 1/2"	18/1 + 28/1	34"+58"	34"+34"	1/8" + 34"	1,811
. M.	(In Ft. Lbs.)	1820	2320	2920	3540	4060	4760	5600	6050
	10 11 12	124 93 68	174 134 103	234 183 145	296 235 188	346 276 222	416 334 270	500 402 329	440 360
	13 14	50 35	79 60	114 91	152 123	180 148	222 182	272 225	298 248
	15		45	72	100	120 99	152 126	188 158	209 176
n Fee	16 17 18		32	56 43	65 52	81 66	105 87	134 113	150 127
ıni	19				41	53	72	95	108
Length of Span in Feet	20 21 22					42	59 48 39	80 67 55	91 77 65
Lengt	23 24							46 37	54 45
	25 26 27 28 29								
	30			1					

	Depth			8′′ T	ILE +					
		5'	'' Joists	25" C-C					. per Sq.	
	Area of Steel	.50 Sq. In.	.64 Sq. In.	.78 Sq. In.	.95 Sq. In.	1.12 Sq. In.	1.34 Sq. In.	1.53 Sq. In.	1.76 Sq. In.	2.00 Sq. In.
		1,5,1	12,1	188	100	34"+34"	1/8" + 34"	1,00	181	1,'
So	Size of quare Bars	+	+	+	+	+	+	+	+	+
ьq	quare Dars	1/2" + 1/2"	100	18	34"	%4 ,4	128	182	1,1	1,
. M.	(In Ft. Lbs.)	2860	3580	4340	5220	6120	7160	8160	9250	9770
	10	219	291	367	455					
	11 12	169 131	229 189	291 233	364 295	438 358	526 432	502		
	13 14	102	145 115	189 155	242 199	295 245	358 299	418 351	480 405	511 431
	15	60	91	126	165	205	251	297	343	367
et	16	45	73	102	137	172	213 181	253 218	294 253	315 271
Fe	17 18	32	57 43	83 67	114 94	145 121	155	185	218	234
ni n	19		32	53	78	102	131	161	189	203
Length of Span in Feet	20 21 22			41 31	63 51 41	86 71 59	112 96 81	138 119 102	164 143 124	177 154 135
Length	23 24				38	48 39	68 58	88 75	108 93	118 103
	25					31	48	64	81	89
	26 27 28						39 31	54 45 37	69 60 51	77 67 58
	29							30	43	49
	30								36	42

	GF	Steel-Tile		
Table of Safe	Live	Loads in Pounds	per S	q. Ft.

	Depth			1	O" TILE	+ 3" CC	NCRET	E		
		5'	Joists	25" C-C	Wei	tht of Sla	b and Joi	st 74 lbs.	per Sq. I	t.
	Area of Steel	50 Sq. In.	.64 Sq. In.	.78 Sq. In.	.95 Sq. In.	1.12 Sq. In.	1_34 Sq. In.	1.53 Sq. In.	1.76 Sq. In.	2.00 Sq. In
	Size of uare Bars	12"+12"	58" + 12"	189 1 + 2811	14"+58"	34"+34"	18" + 34"	78" + 78"	"87+ "1	1 +1
R. M.	(In Ft. Lbs.)	3460	4330	5220	6350	7440	8800	10020	11500	13800
	10 11	272 212	359 284	448	4.53					
	12	166	556	357 288	451 367	442	538			
	13 14	131 102	182 147	235 192	302 250	366 305	447 376	521 439	512	
	15	80	118	158	208	256	317	372	436	494
in Fee	16 17 18	61 46 33	96 76 59	130 107 87	174 146 122	216 183 156	270 231 198	319 274 236	376 324 281	426 368 322
pan	19		46	71	102	132	170	204	245	281
Length of Span in Feet	20 21 22 23		34	56 44 34	85 70 57	112 94 80	146 126 108	176 154 134	213 187 164	246 216 190
I	24				46 36	66 55	93 79	116 100	144 126	168 148
	25 26					45 36	67 56	87 75	110 96	131 116
	27 28 29						46 38	65 54 46	84 73 63	102 89 78
	30							38	54	68

	Depth			1	2" TILE	+ 3" CC	ONCRET	E		
		5'	Joists.	25" C-C	Wei	ght of Sla	h and Joi	st 82 lbs.	per Sq. I	ět.
	Area of Steel	50 Sq. In.	64 Sq. In.	.78 Sq. In.	.95 Sq. In.	1 12 Sq. In.	1.34 Sq. In.	1.53 Sq. In.	1.76 Sq. In.	\$_00 Sq. In.
	Size of pare Bars	12"+12"	28"+3"		3 1 1 + 2 1 1	3,"+3,"	78" + 84"	"87+"87	1" + 7,"	"1 + "1
R. M.	In Ft. Lbs.)	4080	5140	6250	7500	8830	10520	11920	13700	15580
Length of Span in Peet	10 11 12 13 14 15 16 17 18 19 20 21 22 23 24	326 255 202 160 126 99 77 59 44 31	432 342 274 221 180 146 118 95 76 60 46 34	543 434 353 288 237 196 162 134 111 91 74 60 47 36	538 438 362 300 252 210 178 150 126	580 440 368 310 262 223 190 162 139 118 101 85	542 455 387 329 282 243 210 181 157 136 116	528 448 384 331 287 249 216 189 165 144	526 453 392 341 298 260 228 200 177	527 458 400 350 307 271 240 212
	25 26 27 28 29				38 29	59 49 39 31	86 74 62 52 43	109 95 82 70 60	156 137 121 106 93 81	189 168 148 132 117 103

	GF	Steel-Til	е		
Table of Safe	Live	Loads in Po	unds per	Sq.	Ft.

Depth			6"	TILE +	- 31 ₂ " CO	ONCRET	`E		
	5	" Joists	25" C-C	Weig	ht of Slat	and Jois	t 66 lbs.	per Sq. F	
Area of Steel	50 Sq. In.	. 64 Sq. In.	.78 Sq. In.	.95 Sq. In.	1.12 Sq. In.	1.34 Sq. In.	1.53 Sq. In.	1.76 Sq. In.	2.00 Sq. In
Size of Square Bars	1,2" + 1,2"	5/8" + 1/2"	28" + 58"	34"+58"	34"+34"	78"+84"	18/1 + 1/8/1	1" + 7%"	"1 + "1
R. M. (In Ft. Lbs.)	2380	3010	3640	4340	5070	6000	6780	6970	7270
10 11 12 13 14 15 16 17 18 19 20 21 22 28 24 25 26 27 28 29	172 131 99 75 57 41	235 183 143 112 88 68 52 38	298 235 187 150 120 96 60 46 35	368 292 234 190 155 127 103 84 68 54	441 354 286 234 192 160 132 109 90 74 61 49 39	129 350 289 240 200 168 142 119 100 84 70 58 47 38	494 405 335 280 235 198 168 143 122 103 87 74 62 52	510 419 347 290 244 206 150 127 108 92 78 66 55	535 440 365 306 258 218 186 159 136 116 99 84 72 61

	Depth			8"		- 312" CC				
		5'	Joists	25" C-C		ht of Slab				
	Area of Steel	50 Sq. In.	64 Sq. In.	78 Sq. In.	.95 Sq. In.	1.12 Sq. In.	1 34 Sq. In.	1.53 Sq. In.	1,76 Sq. In.	2,00 Sq. In
		20,5	12.1	200	+	1	1 P	18	150	1
	Size of	1	+	+	101	+	+	+	+	+
Sq	uare Bars				,		100			
		1.2"	188	1 × 2	14	17,7	100	1,8/2	1,	1,
. M.	(In Ft. Lbs.)	3020	3800	4590	5500	6430	7570	8600	9870	10650
	10	229	307	386	477					
	1.1	177	241	305	382	457	552			
	12	137	191	245	309	372	452	524		
	13	106	151	198	253	307	374 313	437 367	512 432	557 470
	14	81	131	161	208	255	313	367	436	110
	1.5	61	96	131	171	213	263	309	366	400
÷.	16	4.5	75	106	1+1	177	553	263	313	343
子	17	35	58	86	117	149	189	224	269	295
=	18		4.4	68	97	125	161	193	531	256
Length of Spanin Feet	19		32	54	79	105	137	165	500	222
ď	20			42	64	88	116	142	174	193
9ani C	21			31	52	73	99	155	151	169
4	55				41	60	83	104	131	147
E E	23 24				31	48 38	70 58	89 76	113 98	129
2	2+		1			00	00	10	00	114
	25						4.8	65	85	97
	26						39	54 45	73 62	85 79
	27 28							37	53	63
	29								44	59
	30								37	45

	7	Sable o	of Safe		Steel-		ds per	Sq. Ft		
	Depth	1			TLE + S		_			
		5	" Joists	25" C-0	We	ight of Sl	ab and J	oist 80 H	os, per Sq	. Ft.
	Area of Steel	50 Sq. In.	.64 Sq. In.	78 Sq. In.	.95 Sq. In.	1 12 Sq. In.	1.84 Sq. In	1.58 Sq. In.	1,76 Sq. In.	2.00 Sq. In.
	Size of uare Bars	12"+12"	S * " + 12"	+ . *	34"+58";	" + 3 t " + 3 t "	78"+83"	" + 7 " " + 7 " " "	1" + 1,"	,,1 + ,,1
R. M.	(In Ft. Lbs.)	3580	\$60 <u>0</u>	5550	6650	7800	9510	10150	11900	13500
	10 11 12 13	278 216 168 182 103	380 300 239 192 154	475 380 305 249 203	470 381 314 259	469 381 318	560 465 890	540 454	527	610
Levelh of Soon in Feet	1.5 16 17 1.8 19	79 60 44 31	194 100 79 62 47	166 138 112 91 73	216 180 150 125 104	266 224 190 161 136	280 280 289 204 175	385 328 281 242 210	449 385 332 288 250	520 446 387 336 294
In Ironal of	54 54 55 50		9.5	59 46 85	86 71 57 46 35	115 97 81 67 56	150 128 110 94 80	181 156 136 117 101	217 190 166 145 126	257 226 198 175 154
	#5 #6 #7 ** **					4.5 3.5	67 56 46 37	87 74 63 53 44	110 96 83 72 62	136 120 105 92 80
	100							36	54	70
	Depth				11.10 + 5					
	Arm of Stayl	50 Sq In.	64 So In	25" C C 78 Sq. In	93 Sq. In.	ght of Sh 1 12 Sq. In.	1,34 Sq. In	1 53 Sq. In-	1. 76 Sq. In	2.00 Sq. In.
	Size al save Bara	44.0	24 + 16	34-36	-71+-7	+		31+.31	1.4 + 1.6	10 + 101
0. M.	(In Fig. Lbm.)	4250	55.50	6500	7770	9120	10820	12500	14050	15950
-	10 41 12 12 18 14	937 204 906 164 169 100	\$ \$ 7 9.5 4 9.5 4 9.5 4 9.5 5 18 4	502 450 102 296 244	562 451 172 508	547 452 878	552 465 394	349 467	387	
Spain in You	16 17 16 19	78 30 48 80	3-81 97 77 60	166 1 6 1 1 8 9 6	215 180 151 127	208 227 194 164	336 286 246 212	\$00 344 295 255	462 398 347 302	536 465 405 MM4
Lingshot Span	80 0.1 86 8.6 8.6 6.5		95	7.5 3.9 4.0 15.5	1.06 88 72 58 47	140 118 101 84 70	188 188 186 117 100	284 196 170 148 148	201 201 202 178 156	\$10 \$74 \$4\$ \$14 189
	90 90 87 99 90				#6	58 47 87	8.5 73 61 30 41	112 97 84 72 00	127 120 105 112 112	168 148 161 116 102
	. 200						09	are	GK.	89

Length and Number of GF Steel-Tile for Various Spans

Clear	30" 'File number required	35" Tile number required	End Caps number required	Clear Span	30" Tile number required	35" Tile number required	End Caps
10'-0''	4	0	2	20'-0''	0	7	2
10'-3"	3	1	2	20'-3''	6	2	2
10'-6"	3	1	2	20'-6''	5	3	2
10'-9''	2	2	2	20'-9''	4	4	2
11'-0''	1	3	2	21'-0''	4	4	2
11'-3''	1	3	2	21'-3''	3	5	2
11'-6''	0	4	2	21'-6''	3	5	2
11'-9''	0	4	2	21'-9''	2	6	2
12'-0''	5	0	2	22'-0''	2	6	2
12'-3"	5	0	2	22'-3''	1	7	2
12'-6"	4	1	2	22'-6''	0	8	2
12'-9''	4	1	2	22'-9''	0	8	2
13'-0''	3	2	2	23'-0''	6	3	2
13'-3''	3	2	2	23'-3''	5	4	2
13'-6''	2	3	2	23'-6''	4	5	2
13'-9''	2	3	2	23'-9''	4	5	2
14'-0''	1	4	2	24'-0''	3	6	2
14'-3''	0	5	2	24'-3''	3	6	2
14'-6''	0	5	2	24'-6''	2	7	2
14'-9''	6	0	2	24'-9''	1	8	2
15'-0"	5	1	2	25'-0''	1	8	2
15'-3''	4	2	2	25'-3''	0	9	2
15'-6"	4	5	2	25'-6"	0	9	2
15'-9''	3	3	2	25'-9''	6	4	2
16'-0''	3	3	2	26'-0''	5	5 5	2
16'-3''	2	4	2	26'-3''	5	6	2
16'-6''	1	5	2	26'-6''	4	7	9
16'-9"	1	5	2	26'-9"	3	7	9
17'-0''	0	6	2	27'-0"	3 2	8	9
17'-3''	0	6	2	27'-3''		9	9
17'-6"	6	1	2	27'-6'' 27'-9''	1	9	2
17'-9''	5	2	2	28'-0''	0	10	3
18'-0''	4	3	2	28'-3''	0	10	9
18'-3"	4	3	2	28'-6"	6	5	9
18'-6''	3	4	2	28'-9"	5	6	9
18'-9''	3	4	2	29'-0''	4	7	,
19'-0"	2	5	2	29'-3''	4	7	5
19'-3''	1	6	2	29'-6"	3	8	3
19'-6"	1	6 7	2	29'-9''	3	8	
19'-9"	0	1	2	30'-0''	2	9	

Properties of Steel-Tile Floors

2" OF CONCRETE ABOVE STEEL-TILE

3" OF CONCRETE ABOVE STEEL-TILE

Joists	Jenter Is es	Size Steel-Tile 6"	8"	10"	12"	Joists	enter 8	Size Steel-Tile	6''	8"	10''	12"
th of		Aver'ge weight per square foot 45.8	51 6	58.5	65.6	h of Inch		Aver'ge weight per square foot		63.5	70.5	77.6
N id	ā	Cu. ft. of Concrete per sq. ft. of floor .31	0 .35%	.398	.447	W id⊓		Cu. ft. of Con- crete per sq. ft. of floor		.435	.481	.53
4		Core Area of Section 54.2	58.4	60.8	62.2	4	24	Core Area % of Section	48.9	53.1	56.0	58.0
		Aver'ge weight per square foot 47.9	54.5	62.0	69.9			Aver'ge weight per square foot		66.5	74.0	81.9
5	25	Cu. ft. of Concrete per sq. ft. of floor .32	5 .372	.423	.477	5	25	Cu. ft. of Concrete per sq. ft. of floor		.455	.505	.559
		Core Area % 52.0	56,0	58.3	59.7			Core Area % of Section	46.3	50.9	53.9	55.7

21/2" OF CONCRETE ABOVE STEEL-TILE

31/2" OF CONCRETE ABOVE STEEL-TILE

70	15												
Joists	Sente ts es	Sīze Steel-Tile	6''	7''	10''	12"	Joists	s senter	Size Steel-Tile	6''	8"	10''	12"
in Inch	or to C of Jois n Inch	Aver'ge weight per square foot Cu, ft, of Con-	51.8	57.6	64.5	71.6	th of . I Inch	1200	Aver'ge weight per square foot	63.8	69.6	76.5	83.6
Wid	Cent	crete per sq. ft. of floor		.394	.438	.488	bi N	Cente	Cu. ft. of Con- crete per sq. ft. of floor		.477	.524	.57:
4	24	Core Area % of Section	51.0	55.6	58.2	60,0	4	54	Core Area % of Section	45.8	51.0	54.1	56.3
		Aver'ge weight per square foot		60.5	68.0	75.9			Aver'ge weight per square foot		72.5	80.0	87.9
5	25	Cu. ft. of Concrete per sq. ft. of floor		.413	.464	.518	5	25	Cu. ft. of Concrete per sq. ft. of floor		.497	.547	.601
		Core Area To of Section	19,0	53.4	55.9	57.6			Core Area % of Section	44.0	19.0	51.6	54.0

Steel-Tile are Economically Shipped

The tables of weight below show conclusively the economy in shipping Steel Tile as well as handling it on the job.

Both the Steel-Tile and the End-Tile nest snugly, taking up the minimum space, and stacks are of such shape that the crates are easily handled.

 Λ tapered rod serves to separate the Steel-Tile when needed and it is good practice to keep them stacked until ready to use.

Width of Steel-Tile at bottom, exclusive of flange, 20".

STEEL-TILE

Size	Approx. Per 100	Weight Pe 100 Lineal				
	30" long	35" long	Fret			
6 ⁽⁴⁾ 8'' 10'' 12''	430 470 610 660	500 550 710 770	171 188 243 264			

END-TILE

Size	Approx, Weight Per 100 Pieces
6"	130
8"	160
10"	190
12"	230

General Theory and Working Formulas for Reinforced Concrete

This article is not intended as an elementary treatise on Reinforced Concrete, but rather to show the application of the general theory and formulas on which the foregoing tables are based. It is assumed that those who use this Handbook understand the general principles underlying Reinforced Concrete design.

The accompanying formulas and computations are based on the following assumptions:

- 1. The adhesion between the concrete and steel is sufficient to make the two materials act together.
 - 2. The Stress Strain curve for concrete in compression is a straight line.
 - 3. The concrete carries no direct tension.
 - 4. The ratio of the Modulus of Elasticity of Steel to that of 1:2:4 Concrete is 15.

The sketches illustrate graphically the principles embodied in the above assumptions.

The following notations have been used throughout this Handbook.

Beams and Slabs

f. = the unit fiber stress of the steel.

 f_c = the unit fiber stress of the concrete.

E, = the modulus of elasticity of the steel.

E_e = the modulus of elasticity of the concrete in compression.

n = the ratio of E_s ÷ E_c.

T = the total tension in the steel at a section of the beam or slab.

C = the total compression on the concrete at a section of the beam or slab.

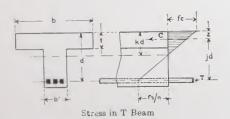
M = the bending moment in inch-pounds.

M, = the moment of resistance of the steel in inch-pounds.

 M_c = the moment of resistance of concrete in inch-pounds.

b = in inches the breadth of a rectangular beam or slab under consideration, or the width of flange of a T beam.

b' = the width of stem of a T beam in inches.



d = the distance from the top of the compressive face of the concrete to the center of gravity of the steel.

Stress in Rectangular Beam

k = the ratio of depth of the neutral axis to the effective depth d.

= the ratio of lever arm of resisting couple to depth d.

A. = the cross-sectional area of steel.

p = the percentage of steel—equal to $\frac{A_s}{bd}$

z = the distance from top of concrete to the centroid of compression area.

jd = the arm of the resisting couple in inches; = d-z.

Shear

V = Total Vertical Shear at given section.

 $v = \frac{V}{bid}$ = unit Vertical Shear at given section in lbs. per sq. in.

v' = v-60 = Shear in lbs. per sq. in. carried by Stirrups.

A_v = Sect. Area of one Stirrup Rod in sq. in.

S = Horizontal Spacing in inches of Stirrups at given section.

Bending Moments

Slabs and girders continuous over supports act as continuous beams and must be provided with reinforcing at these points to take care of the negative bending moment. Provision for the negative bending moment over the supports materially reduces the positive bending moment at the center of span. It is considered good practice to use the following values:

Freely supported at both ends. M = 1/8 W1 Freely supported at one end, and continuous at the other. M = 1/10 W1

W = total load on the slab or beam under consideration.

1 = span in feet if M is to be expressed in foot-pounds.

1 = span in inches if M is to be expressed in inch-pounds.

Shear

Anything like a thorough analysis of shearing stresses would be far beyond the scope of this Handbook. Therefore, we will proceed on the assumption that the concrete is capable of resisting a unit shearing stress of 60 pounds per square inch, and that any shear in excess of this amount must be taken up by vertical stirrups.

Allowing a unit shearing stress in the steel of 12000 pounds per square inch, we have for the required horizontal spacing of the stirrups at any given section:

$$S = \frac{2A_{v} \times 12000}{\text{by}}....(1)$$

For beams uniformly loaded, stirrups should in general be spaced at "S" inches on centers for a distance from the support equal to about one-sixth of the span. From this point the spacing should be gradually increased for another one-sixth span. Stirrups spaced farther apart than a distance equal to jd cannot be considered effective, but may be employed as an aid in holding the main beam reinforcement.

Formulas

Rectangular Beams and Slabs

Location of neutral axis— $kd = (\sqrt{2pn + (pn)^2} - pn)d$. (2) Arm of Resisting Couple— $jd = (1 - \frac{1}{3}k) d$. (3)

Fiber Stresses—.... $f_s = \frac{M}{A_s j d} = \frac{M}{p j b d^2}$ (4)

 $f_c = \frac{2M}{jkbd^2} = \frac{2pf_e}{k}.$ (5)

Percentage of Steel— $p = \frac{A_s}{bd} \cdot \dots$ (6)
For Balanced Reinforcement— $p = \frac{\frac{1}{2}}{f_c} \left(\frac{f_s}{nf_c} + 1 \right)$ (7)

(By "Balanced Reinforcement" is meant the percentage of steel at which both steel and concrete are working at the allowable unit stresses.)

Depth "d" $d = \sqrt{\frac{M}{Rb}}$ in which $R = f_o pj$(8)

For 12" width of slab or rectangular beam, assuming $f_{\rm s}\!=\!16000$ and $f_{\rm e}\!=\!650$,

 $d = .028 \sqrt{M} \dots (9)$

Steel required $A_a = \frac{M}{idf_a} = pbd.$ (10)

Formulas for T Beams

If the Neutral Axis falls in the Flange, use the foregoing formulas for Rectangular Beams; if in the Stem, the following will apply:

Location of Neutral Axis— $kd = \frac{2ndA_s + bt^2}{2nA_s + 2bt}.$ (11)

Location of Resultant Compression— $z = \frac{t(3kd-2t)}{3(2kd-t)}$(12)

Arm of Resisting Couple— jd=d-z....(13)

Fiber Stresses— $f_c = \frac{Mkd}{bt (kd - \frac{1}{2}t) jd} \cdot \dots (14)$

 $f_{\scriptscriptstyle 0} = \frac{M}{A_{\rm s} j d} \cdot \dots (15)$

Approximate formula— $T = \frac{M}{d - \frac{1}{2}t}$ (16)

Stayed Columns

Notation

P = safe load or total vertical load.

f_c = allowable unit stress for concrete in direct compression.

A_c = total cross sectional area of concrete in compression.

 $n = E_a/E_c = 15.$

A, = total cross sectional area of vertical steel.

Hooped Columns

Notation

P = safe load or total vertical load.

f_c = allowable unit stress in the concrete within the hooping.

A_c = cross sectional area of the concrete within the hooping.

 $n = E_a/E_c = 15.$

A. = total cross sectional area of vertical steel.

A' = cross sectional area of one rod of hooping.

h = inside diameter of hooping.

f. = allowable unit tension for hooping steel.

s = pitch or vertical spacing of spiral hooping.

 $P = \left(A_c + nA_s\right) \left(f_c + 4.8 \frac{f_s/s \times A'_s}{h}\right) \dots (18)$

Reinforced Concrete Footings

With but slight variation the design of reinforced concrete footings follows the principles and formulas already laid down for rectangular beams and slabs.

The design of the footing illustrated below is based on the following assumptions:

1. The load is transmitted from the column to the footing along lines a,a, having a slope of 1:3 and extending from every point in the perimeter of the column to the base of the footing.

2. The load is carried to the soil by two cross girders whose effective depth = d; length

= B and width = b.

3. The load causing the bending moment in each of these cantilever cross girders is equal to $\frac{1}{4}$ of the load obtained by deducting the load on the area b×b from the total column load.

4. This load acts with a lever arm L.

The problem, therefore, is simply to proportion the cantilever cross girders to resist the bending moment set up by this load.

Notation

A = required area of footing in square feet.

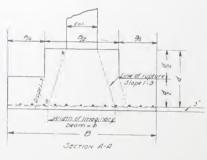
W = total column load in pounds.

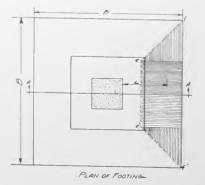
S = allowable soil pressure in pounds per square foot.

 $P = \frac{1}{4}$ of load carried by footing outside the line of rupture.

= load on area efe'f'.

L = distance from side of column to center of gravity of area efe'f'.





$$\mathbf{A} = \frac{\mathbf{W}}{\mathbf{S}} \cdot \dots \tag{19}$$

$$M = PL....(20)$$

To find the required depth of footing and the amount of steel reinforcement, apply the formulas for rectangular beams.

Footings should be so proportioned that the shear on any plane of rupture shall not exceed 30 pounds per square inch.

Computation of GF Steel-Tile Slab Using Formulas

Span 20'-0" L. L. 80 lbs. $f_a = 16000$ $f_c = 650$ n = 15.

Assume 12" GF Steel-Tile and 2" Conc. Slab. Joists 25" on centers. (5" wide at bottom.)

$$d = 12\frac{1}{2}$$
".

Load on 1 Joist:

Live Load— Slab and Joist— Flr. Finish— 80 lbs. per Sq. Ft. 70 lbs. per Sq. Ft. 12 lbs. per Sq. Ft.

Ceiling— 10 lbs. per Sq. Ft.

172 lbs. per Sq. Ft. × 2.08 = 358 lbs. per Lin. Ft. of Joist.

Total Load on 1 Joist = 358 x 20 = 7160 lbs.

$$M = \frac{7160 \times 20 \times 12}{10} = 172000 \text{ inch Lbs.}$$

From Formula (10) $A_s = \frac{172000}{11.5 \times 16000} = .93 \text{ Sq. Inches.}$

(In which $11\frac{1}{2}$ " is Approx. Lever Arm = $d-\frac{1}{2}t$)

From Formula (11) We Have kd = $\frac{2 \times 15 \times 13 \times .93 + 25 \times 4}{2 \times 15 \times .93 + 2 \times 25 \times 2} = 3.62''$.

As Kd is Greater Than 2", The Neutral Axis is in the Web, and T Beam Formula Applies

From Formula (12)
$$z = \frac{2(3 \times 3.62 - 4)}{3(2 \times 3.62 - 2)} = .87$$

From Formula (13) jd = 12.50 - .87 = 11.63.

Actual Steel Req'd—Formula (10) $A_s = \frac{172000}{11.63 \times 16000} = .924 \text{ Sq. Inches.}$

Use—
$$1-\frac{34}{8}$$
 sq. Bar = .56 Sq. Inches.
 $1-\frac{5}{8}$ sq. Bar = .39 Sq. Inches.
.95 Sq. Inches

From Formula (14) $f_c = \frac{172000 \times 3.62}{24 \times 2 \times 2.62 \times 11.63} = 409 \text{ lbs Per Sq. In.}$ Which is Well Below the 650 lbs. Allowed

Maximum End Shear = ½ Total Load on Joist = 3580 Lbs.

Unit Shear = $\frac{3580}{11.63 \text{ x } 6}$ = 51.3 Lbs. per Sq. Inch.

(6" = Average Thickness Of Joist)

Table of Weights of Materials and Loads in Storage Warehouses

MATERIAL	Weights per cu. ft. of space Pounds	Weights per sq. ft. of floor Pounds	Recommend ed live loads in lbs. per sq. ft.
GROCERIES, WINES, LIQUORS, ETC.			
Beans in bags	40	320	250 to 300
Canned goods in cases	58	348	250 to 300
Coffee in bags	39	312	250 to 300
Flour	40	200	250 to 300
Molasses	48	240	250 to 300
Rice	58	348	250 to 300
Salt in bags	70	350	250 to 300
Sugar in barrels	43	215	250 to 300
Tea in chests	25	200	250 to 300
Wines and liquors in barrels	38	228	250 to 300
DRY GOODS—COTTON, WOOL, ETC.			
Burlap in bales	43	258	200 to 250
Cotton in bales, compressed	18	144	200 to 250
Cotton goods in cases	28	224	200 to 250
Hemp, manila	30	240	200 to 250
Jute	41	328	200 to 250
Linen goods	30	240	200 to 250
Wool in bales, not compressed	13	104	200 to 250
Wool in bales, compressed	48	104	200 to 250
Woolen goods in cases	27	216	200 to 250
BUILDING MATERIALS, HARDWARE, ETC.			
Portland Cement	73	438	300 to 400
Small Hardware	30 to 65	300 to 400	300 to 400
Sheet Tin in boxes	278	556	300 to 400
Wire coils	75	450	300 to 400
DRUGS, PAINTS, OILS, ETC.		100	000 60 100
Alum in barrels	33	198	200 to 300
		010	200 to 300
Glycerine in cases	52	312	200 10 300
Glycerine in cases. Linseed oil in drums	52 45	180	
Unseed oil in drums			200 to 300
Linseed oil in drums Rosin in barrels Soda, caustic, in iron drums	45	180	
Unseed oil in drums Rosin in barrels Soda, caustic, in iron drums Sulphuric acid	45 48	180 288	200 to 300 200 to 300
Unseed oil in drums Rosin in barrels Soda, caustic, in iron drums Sulphuric acid White lead in cans	45 48 88	180 288 294	200 to 300 200 to 300 200 to 300 200 to 300
Unseed oil in drums Rosin in barrels Soda, caustic, in iron drums Sulphuric acid White lead in cans White lead, dry	45 48 88 60	180 288 294 100	200 to 300 200 to 300 200 to 300
Einseed oil in drums Rosin in barrels Soda, caustic, in iron drums Sulphuric acid White lead in cans White lead, dry Red lead and Litharge	45 48 88 60 174	180 288 294 100 610	200 to 300 200 to 300 200 to 300 200 to 300 200 to 300
Einseed oil in drums Rosin in barrels Soda, caustic, in iron drums Sulphuric acid White lead in cans White lead, dry Red lead and Litharge MISCELLANEOUS	45 48 88 60 174 86	180 288 294 100 610 408	200 to 300 200 to 300 200 to 300 200 to 300 200 to 300 200 to 300
Einseed oil in drums Rosin in barrels Soda, caustic, in iron drums Sulphuric acid White lead in cans White lead, dry Red lead and Litharge MISCELLANEOUS Glass and chinaware in crates	45 48 88 60 174 86	180 288 294 100 610 408	200 to 300 200 to 300 200 to 300 200 to 300 200 to 300 200 to 300
Einseed oil in drums Rosin in barrels Soda, caustic, in iron drums Sulphuric acid White lead in cans White lead, dry Red lead and Litharge MISCELLANEOUS Glass and chinaware in crates Hides and leather	45 48 88 60 174 86 132	180 288 294 100 610 408 495	200 to 300 200 to 300
Enseed oil in drums Rosin in barrels Soda, caustic, in iron drums Sulphuric acid White lead in cans White lead, dry Red lead and Litharge MISCELLANEOUS Glass and chinaware in crates Hides and leather Paper, newspaper and straw board	45 48 88 60 174 86 132	180 288 294 100 610 408 495	200 to 300 200 to 300
Einseed oil in drums Rosin in barrels Soda, caustic, in iron drums Sulphuric acid White lead in cans White lead, dry Red lead and Litharge MISCELLANEOUS Glass and chinaware in crates	45 48 88 60 174 86 132	180 288 294 100 610 408 495	200 to 300 200 to 300

NOTE—The figures in the column under weights per sq. ft. of floor are based on the height to which it is convenient and practicable to pile the different kinds of material, viz.: Beans in bags can be piled to 8', salt to 5', cement to 6', etc.

Rectangular Wooden Beams

One Inch Thick
Allowable Uniform Load in Pounds
American Railway Engineering Assn. Formulas

Span	1			Depth	of bean	-inch	es		
in Feet	2	4	6	8	10	12	14	16	18
2	267								
3	178								
4	133	533							
5	107	427							
6	89	356	800						
6 7	76	305	686						
8	67	267	600	1067					
9		237	533	948					
10	1	213	480	853	1333				
11		194	436	776	1212	1745			
12		178	400	711	1111	1600			
13			369	656	1026	1477	2010		
14			343	610	952	1371	1867		
15			320	569	889	1280	1742	2276	
16			300	533	833	1200	1633	2133	
17				502	784	1129	1537	2008	2541
18				474	741	1067	1452	1896	2400
19				449	702	1011	1375	1796	227
20				427	667	960	1307	1707	2160
21					635	914	1244	1625	2057
22					606	873	1188	1552	1964
23					580	835	1136	1484	1878
24					556	800	1089	1422	1800
25	3					768	1045	1365	1728

The above table is for Douglas Fir. To obtain allowable uniform load for Longleaf Pine, add 1/12th or 8% to above figures. For Shortleaf Pine, Hemlock or White Oak deduct 1/12th or 8% from above figures. For White Pine deduct 1/4th or 25%.

Square Wooden Columns Safe Loads in Thousands of Pounds

	Length			Side	of Squ	iare—I	nches			
	Feet	4	6	8	10	12	14	16	18	20
	6	14.6								
Longleaf	8	12.5	34.3							
Pine and	10	10.4	31.2	62.4						
White	12		28.1	58.2						
Oak	14			54.1	93.6					
	16	1	1	49.9	88.4	137.3				
	18			45.8	83.2	131.0	189.3	2*0 0	212.0	000 0
	20			41.6	78.0	124.8	182.0	250.0	316.0	390.0
	6	13.4								
Douglas	8	11.5	31.7							
Fir and	10	9.6	28.8	57.6						
Western	12		25.9	53.8						
Hemlock	14		23.0	49.9	86.4					
	16			46.1	81.6	126.7	104.0			
	18			42.2	76.8	121.	174.7	200 4	201 6	960 0
	20			38.4	72.0	115.2	168.0	230.4	291.6	360.0

To obtain Safe Load on Shortleaf Pine or Spruce Columns deduct 7% from table for Douglas Fir. For White Pine or Tamarack deduct 15%.

Weight and Area of Square and Round Bars

Size		in Lbs. foot	Area square		Size		in Lbs.	Ares			
1/4"/5"/16 3/"/16 1/2"/2 9 1/5 //"/	.213 .332 .478 .651 .850 1.076 1.328	.167 .261 .376 .511 .668 .845 1.043	.0625 .0977 .1406 .1914 .2500 .3164 .3906	.0491 .0767 .1105 .1503 .1963 .2485 .3068	111// 16/3// 13// 15// 15//	1.607 1.913 2.245 2.603 2.988 3.400	1.262 1.502 1.763 2.044 2.347 2.670	.4727 .5625 .6602 .7656 .8789 1.0000	.3712 .4418 .5185 .6013 .6903 .7854		

Table No. 14
Cubic Yards Concrete Required for Beams, Columns and Slabs

												C	OLU.	MNS		1	SL	ABS	
												Sąu	ARE	R	OUND				
C	CUBIC YARDS OF CONCRETE FOR BEAMS 100 FT. LONG Width 4 5 6 7 8 9 10 11 12									of Square or	c Yds. per Ft.	Weight per Ft. Height and Area Section	c Yds. per Ft.	Weight per Ft. Height and Area Section	rness		ic Yds. Per Sq. Ft.	Weight Per Sq. Ft.	
Width	Inch I			7 Inch	8 Inch	9 Inch	10 Inch	11 Incl	n I	12 nch	Side of S Diam. o	Cubic Height	Weigh and Ar	Cubic Height	Weight and Ar	Thickness		Cubic 100 Sq.	Weig
5" 6" 6" 7" 8" 9" 10" 11" 12" 13" 15" 15" 15" 15" 12" 12" 12" 12" 12" 12" 12" 12" 12" 12	720 823 1 9261 1 1 329 1 1 132 1 1 235 1 1 337 1 1 440 1 1 543 1 1 646 2 1 749 2 1 852 2 2 1 955 2 2 1 612 2 2 263 2 2 469 3 2 469 3 2 572 3 2 674 3 2 778 3 2 881 3	157 1 1 286 1 1 4 4 5 1 5 4 4 3 1 5 5 4 3 1 6 7 2 2 8 6 1 2 9 2 9 2 2 0 5 8 2 2 1 8 6 2 2 0 5 8 2 2 1 8 6 2 2 6 5 7 2 1 8 6 2 2 6 5 7 2 6 1 8 6 2 2 6 1 8 6 2 6 1 8 6 2 6 1 8 6 2 6 1 8 6 2 6 1 8 6 2 6 1 8 6 2 6 1 8 6	235 389 543 697 852 0066 161 315 468 624 778 240 394 240 394 475 630 012 167 321 475 630 091 248 401 249 401 249 401 249 401 401 401 401 401 401 401 401 401 401	1 440 1 620 1 801 1 1 801 1 1 981 1 2 161 1 2 2 521 2 701 3 3 061 3 3 421 3 3 601 3 3 781 3 3 421 4 4 321 4 4 501 5 5 401 5 5 5 781 5 5 781 6 6 122 6 301	2 2 675 2 2 469 2 2 675 2 3 086 3 2 92 3 3 704 3 909 4 115 4 526 4 732 4 732 4 732 5 5 761 6 379 6 5 790 6 790 6 790 6 790 6 790	2. 546 2. 7788 2. 0093 3. 241 3. 4723 3. 935 4. 1674 4. 398 4. 630 4. 630 4. 630 4. 630 5. 555 5. 786 6. 0188 6. 481 6. 713 6. 713 6. 713 6. 713 7. 7	2 829 3 086 3 343 3 601 3 858 4 115 4 630 4 630 5 916 6 173 6 429 6 686 6 944 7 202 7 7 156 7 7 974 8 230 8 487 8 745 8 745	3 1 3 33 3 67 3 96 4 22 4 52 4 52 5 63 5 63 5 63 7 63 7 7 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	95 3 777 444 444 4400 558 66 60 77 77 72 77 758 888 888 888 888 888 895 899 999 999 199 109 109 109 109 109 109 1	.012 321 630 936 247 555 864 173 482 790 099 408 716 023 333 642 951 2568 876 18	6"7" 8" 8" 9" 10" 11" 11" 11" 15" 16" 17" 12" 12" 12" 12" 22" 23" 22" 28" 28" 31" 32" 33" 34" 35" 36" 36" 36" 36"	.263 280 292 315	676 729 784 841 900 961 1024 1089 1156	.194 207 .220 .233 247	855 3 907.9	5" 5½' 6" 6½ 7" 7½ 8½ 9½ 10" 10½ 11"	1 2 2 2 2 2 2 3 3 3 3 3 3	852 .006 .161 .315 .469 .624 .778 .932 .086 .241 .396 .550	

Table No. 16

Quantities of Materials for One Cubic Yard of Rammed Concrete Based on a Barrel of 3.8 Cubic Feet

(Reprinted by permission from Taylor & Thompson's "Concrete, Plain and Reinforced," page 231)

	portic	n n a	Pro	portic	ona	e e e				Perce	ntages	of Vo	ids in	Broke	n Sto	ne or (Grave	1			
	Parts			Volur		Mortar in ercentage of Stone		50%*			15%†	1	4	10% ‡			30% §			20%§	
Cement	Sand	Stone	Packed Cement	Loose	Loose	Volume of Mortar in Terms of Percentage of Volume of Stone	Cement	Sand	Stone	Cement	Sand	Stone	Cement	Sand	Stone	Cement	Sand	Stone	Cement	Sand	Stone
0	0,		bbl.	cu. ft.	cu. ft.	Terr of	bbl.	cu. yd.	cu. yd.	bbl.	cu. yd.	eu. yd.	bbl.	cu. yd.	cu. yd.	bbl.	cu. yd.	cu. yd.	bbl.	cu. yd.	cu. yd.
	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	5 51 6 61	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	3 8 8 3 8 8 3 8 8 5 7 7 6 6 7 7 6 6 7 7 6 6 7 7 6 6 9 5 5 9 5 5 9 5 5 9 5 5 11 11 11 11 11 11 11 11 11 11 11 11	3 . 8 . 7 . 6 . 11 . 4 . 15 . 2 . 2 . 8 . 3 . 8 . 4 . 5 . 5 . 7 . 6 . 9 . 5 . 11 . 4 . 4 . 5 . 5 . 11 . 4 . 15 . 2 . 2 . 8 . 11 . 4 . 15 . 2 . 17 . 1 . 1 . 1 . 1 . 1 . 1 . 1 . 1 .	76 64 49 44 40 75 57 51 47 51 47 51 47 51 48 40 66 65 64 66 68 52 66 68 52 68 68 68 52 68 68 68 52 68	2 27 2 099 1 .94 1 .80 1 .69 1 .59 1 .75 1 .55 1 .55 1 .47 1 .30 1 .32 1 .52 1 .52 1 .32 1 .22 1 .22 1 .22 1 .30 1	0 45 0 0 40 0 36 0 0 44 0 0 57 0 0 46 0 0 57 0 0 46 0 0 57 0 0 54 0 0 50 0 50 0 0 50 0 0 50 0 5	0 72 1 03 1 03 1 03 1 03 1 03 1 03 1 03 1 0	2 . 22 2 2 . 40 1 . 84 1 . 71 1 . 60 1 . 50 0 . 61 1 . 81 1 . 68 1 . 57 1 . 60 1 . 50 0 . 61 1 . 62	0.43 0.38 0.31 0.31 0.41 0.42 0.39 0.36 0.34 0.41 0.42 0.39 0.36 0.40 0.40 0.40 0.40 0.40 0.40 0.40 0.4	0 69 0 0,98 1 .14 0 .65 0 .77 0 86 0 94 0 .91 0 .96 0 .76 0 .88 0 .94 0 .91 0 .92 0 .95 0 .77 0 .84 0 .91 0 .92 0 .93 0 .94 0 .94 0 .95 0 .97 0 .94 0 .97 0 .94 0 .97 0 .9	1.41 1.32 1.25 1.18 1.60 1.49 1.40 1.31 1.24 1.17 1.11 1.11 1.05 0.92 0.88 0.81 0.76 0.71	0.42 0.37 0.33 0.30 0.49 0.44 0.37 0.32 0.40 0.42 0.40 0.37 0.36 0.42 0.40 0.40 0.37 0.36 0.42 0.40 0.40 0.40 0.40 0.40 0.40 0.40	0 67 0 .93 1 .07 0 .63 0 .74 0 .82 0 .90 0 .65 0 .74 0 .81 0 .90 0 0 .90 0 0 .90 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1.766 1.611 1.488 1.377 1.288 1.611 1.488 1.289 1.130 1.061 1.131 1.071 1.010 0.961 0.911 1.131 0.966 0.971 0.870 0.870 0.870 0.688 0.644 0.600	0 39 0 34 0 30 0 27 0 45 0 39 0 34 0 30 30 0 27 0 45 0 42 0 39 0 36 0 34 0 31 0 45 0 45 0 45 0 45 0 45 0 45 0 45 0 4	0 611 00 82 0 94 1 1 00 1 1 00 1 1 00 1 1 00 1 1 00 1 1 00 1 1 00 1 1 00 1 1 00 1 1 00 1 1 0 1	0.58 0.55	0 37 0 32 0 28 0 0 43 0 38 0 38 0 39 0 37 0 32 0 31 0 46 0 42 0 30 30 30 30 30 30 30 30 30 30 30 30 3	0.57 0.84 0.89 0.92 0.95 0.97 0.98 0.99 1.00 0.75 0.63 0.64 0.70 0.63 0.69 0.70 0.73 0.79 0.82 0.63 0.69 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.7

Note-Variations in the fineness of the sand and the compacting of the concrete may affect the quantities 10 per cent in either

Wote—variations in the interess of the Salar Anti-Salar Mote —variations in the interest of the Salar Mote.

*Use 50 per cent columns for broken stone screened to uniform size. †Use 45 per cent columns for average conditions and for broken stone with dust screened out. ‡Use 40 per cent columns for gravel or mixed stone and gravel. §Use these columns for scientifically graded mixtures.



Building Materials

The following materials particularly adapted to fireproof construction are manufactured by The General Fireproofing Company:

Self-Sentering Herringbone Metal Lath Trussit

Steel-Tile for Floors

Key Expanded Metal Lath Expanded Metal Cold Rolled Channels Corner Bead

Wall Ties

The GF trade-mark is a guarantee of quality in the materials themselves and of intelligent service on all building operations.

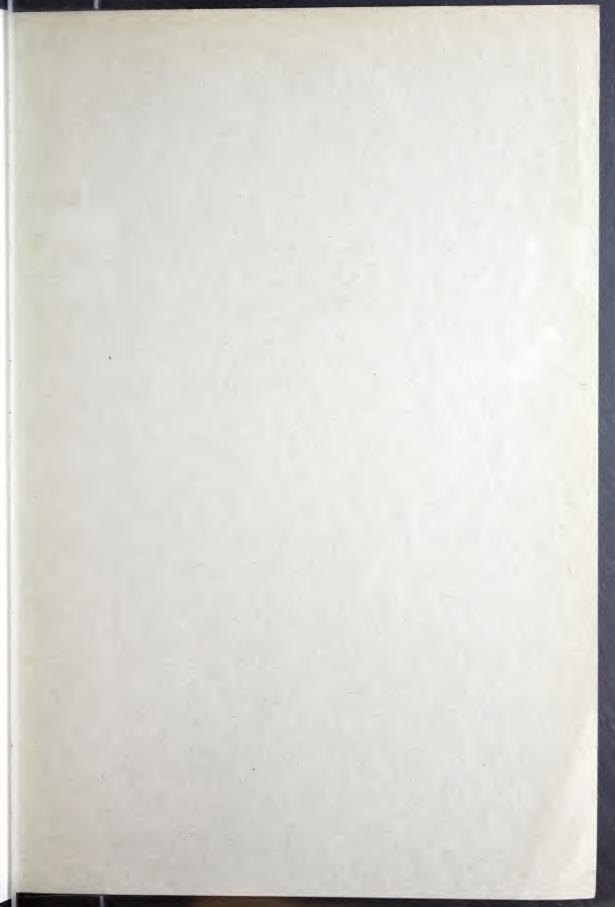
Special literature covering each of the above materials will be sent to anyone interested.

GF Waterproofing Materials

In addition to the structural materials named The General Fireproofing Company produces a full line of Waterproofing Materials.

Write our Waterproofing Service Department for information at any time—their advice will be complete and dependable and, if you wish, in the form of specifications for the work.

The Waterproofing Handbook mailed on request.





STEEL-TILE FLOOR CONSTRUCTION